

EVALUATION OF THE SEISMIC RISK OF BUILDINGS THROUGH VULNERABILITY FUNCTIONS IN AYACUCHO CITY – PERU

EVALUACIÓN DEL RIESGO SÍSMICO DE EDIFICACIONES MEDIANTE FUNCIONES DE VULNERABILIDAD EN LA CIUDAD DE AYACUCHO – PERÚ

Jhuniur Sulca-Huamán ^{1*}, Rick M. Delgadillo ¹

¹ Facultad de Ingeniería de Minas Geología y Civil, Universidad Nacional de San Cristóbal de Huamanga, Ayacucho, Peru

Received: 06/11/2021 Accepted: 08/08/2022

ABSTRACT

Peru is one of the most exposed countries to catastrophic events worldwide, such as floods, debris flows, earthquakes and tsunamis. Particularly, earthquakes are generated by the interaction of the Nazca and South American plates, known as subduction process, and from shallow faults. Likewise, another big problem that Peru is going through is the informality in construction, which, added to a big earthquake, could bring with it great economic losses and human lives. Therefore, these problems have motivated the development of this research, which aims to evaluate the seismic risk of buildings in Ayacucho city - Peru. In this sense, the probabilistic seismic hazard is evaluated using R-CRISIS program, accompanied by geophysical studies like MASW (Multichannel Analysis of Surface Waves) that allow characterizing the ground from shear wave velocities. In addition, the seismic vulnerability of buildings is evaluated using the vulnerability index method, which is based on eleven essential parameters that characterize the susceptibility of buildings to suffer damage after the occurrence of an earthquake. Finally, the seismic risk assessment is carried out through the use of vulnerability functions and the results reflected in representative maps for different seismic scenarios, which is prepared using Geographical Information Systems (GIS). Results show that great economic losses and human lives could be expected from different seismic scenarios in Ayacucho city. Besides, more than 50% of the buildings could suffer strong or severe damage during the occurrence of a rare earthquake whose return period is 475 years.

Keywords: Seismic hazard, vulnerability, seismic risk, earthquake, Ayacucho

RESUMEN

Perú es uno de los países más expuestos a eventos catastróficos a nivel mundial, como inundaciones, flujos de escombros, terremotos y tsunamis. En particular, los sismos se generan por la interacción de las placas de Nazca y Sudamericana, conocido como proceso de subducción, y por fallas someras. Asimismo, otro gran problema que atraviesa el Perú es la informalidad en la construcción, que sumado a un gran terremoto, podría traer consigo grandes pérdidas económicas y vidas humanas. Por lo tanto, estos problemas han motivado el desarrollo de esta investigación, que tiene como objetivo evaluar el riesgo sísmico de las edificaciones en la ciudad de Ayacucho - Perú. En este sentido, la amenaza sísmica probabilística se evalúa mediante el programa R-CRISIS, acompañado de estudios geofísicos como MASW (Análisis Multicanal de Ondas Superficiales) que permiten caracterizar el suelo a partir de las velocidades de las ondas de corte. Además, la vulnerabilidad sísmica de las edificaciones se evalúa mediante el método del índice de vulnerabilidad, el cual se basa en once parámetros esenciales que caracterizan la susceptibilidad de las edificaciones a sufrir daños luego de la ocurrencia de un sismo. Finalmente, la evaluación del riesgo sísmico se realiza mediante el uso de funciones de vulnerabilidad y los resultados se reflejan en mapas representativos para diferentes escenarios sísmicos, los cuales se elaboran utilizando Sistemas de Información Geográfica (SIG). Los resultados muestran que se podrían esperar grandes pérdidas económicas y de vidas humanas en diferentes escenarios sísmicos en la ciudad de Ayacucho. Además, más del 50% de las edificaciones podrían sufrir daños fuertes o severos durante la ocurrencia de un terremoto raro cuyo período de retorno es de 475 años.

Palabras clave: Amenaza sísmica, vulnerabilidad, riesgo sísmico, terremoto, Ayacucho

* Corresponding author:

E-mail: jhuniur.sulca.16@unsch.edu.pe

DOI: <https://doi.org/10.21754/tecnica.v32i2.1377>

1. INTRODUCTION

Throughout history, seismic motions recorded worldwide have evidenced catastrophes of high magnitudes generating considerable damage and even the collapse of structures. This is a latent problem in Peru as it is part of the Pacific Ring of Fire, making it one of the countries with the highest seismicity in the world. In addition, the disorderly growth of buildings in Ayacucho city generates risky conditions that can bring large economic and human losses.

Nowadays, many researchers ([1], [2], [3]) are implementing modern procedures to assess risk in bridges, buildings, dams, as they are subject to multiple hazards such as earthquakes, hurricanes, debris flow, landslides. Likewise, new technological tools are being used to assess seismic vulnerability. For example, [4] implements the use of drones and the photogrammetry technique within a vulnerability study. In addition, Peru has been developing seismic microzoning studies in different cities ([5], [6], [7]). These investigations show the importance and necessity of conducting risk studies, which serve as principal motivation for the present research.

The main objective of this article is to evaluate the seismic risk of buildings in Ayacucho - Peru. This due to the fact that this city is exposed to a latent seismic threat and where informality has become common in the study area with the presence of quite old buildings and with materials that can become extremely vulnerable to a seismic event. Therefore, this research aims to develop seismic zoning, vulnerability and risk maps, which serve both the population and the government in order to anticipate and mitigate possible disasters.

2. BACKGROUND

Many researchers devoted great effort to assess seismic risk of buildings. For instance, Mena [8] evaluated the seismic risk in the urban area of Barcelona city, Spain, where despite not being an area of great seismicity, high damage was obtained for relatively low intensity levels due to the high vulnerability of its buildings.

Angeletti et al. [9] studied the damage in structures caused by an earthquake in Italy, which made it possible to generate vulnerability functions by relating the damage index with the vulnerability index conditioned to the maximum soil acceleration. Due to the importance of these, many functions began to be generated and simulated for different

countries, for example, Barbat et al. [10] in Spain; Maldonado et al. [11] in Colombia; Quispe [12] in Peru.

In Peru, several researchers ([13], [14], [15]) evaluated the seismic hazard over the years considering various seismogenic sources which were updated over time. The results of these research were captured in isoacceleration maps considering different levels. A free consultation Web application was developed taking into account a data density for a 0.10° longitude-latitude grid [15].

In the same way, ([16], [17], [18]) evaluated the seismic risk of a group of buildings in several places in Peru, taking into account the vulnerability index methodology and the probabilistic evaluation of seismic hazard. Furthermore, they carried out geophysical studies to know the dynamic behavior of the soil. In this way, the performance and robustness of the methodology have been shown in many study areas. For example, [19] adapts the vulnerability index method for seismic risk studies in the city of Mérida, Venezuela.

3. METHODOLOGY

A total of 2078 buildings located in Ayacucho city have been evaluated (the southern central part of Peru). The study sample includes adobe buildings, confined masonry and reinforced concrete, where the results of seismic vulnerability were analyzed taking into account each structural typology.

Given the large number of buildings that the study population represents, the seismic vulnerability of the most representative buildings of each block was analyzed, allowing representative maps to be generated. Likewise, in the case of seismic hazard, the maximum acceleration of the ground has been evaluated for four seismic scenarios, placing greater emphasis on the rare earthquake as this is the design earthquake according to the E.030 Seismic Design Standard (RNE of Peru). Additionally, as part of the seismic hazard study, a geophysical test called MASW were carried out throughout the study area, which together with information collected from various soil studies, allowed the generation of a seismic geotechnical zoning map of the Ayacucho city. The seismic hazard and vulnerability results in buildings allowed to know the level of damage, being represented by risk maps for different seismic scenarios.

4. STUDY AREA

Ayacucho is one of the most important cities in Peru, located on the eastern slope of the Andes

Mountain range at an altitude of 2,760 meters above sea level ($13^{\circ}09'37''S$ and $74^{\circ}13'33''W$, UTM coordinates 583899.8 m E and 8545010.6 m N, DATUM WGS84, zone 18, southern hemisphere). The study area is Ayacucho city, includes both the urban and peripheral areas of urban expansion of the Ayacucho district. This district comprises a large territorial extension, considered as the capital of the sixteen districts that make up the province of Huamanga in the department of Ayacucho. Given the great extension of its territory, the study area does not include rural areas because small populated centers are scattered at far distances throughout the district (see Fig. 1).

The study area comprises a total of 7,697 km² with the following limits: the east it borders the district of Andrés Avelino Cáceres Dorregaray, Jesús Nazareno and San Juan Bautista; to the west, with the Huaschura Populated Center; to the North, with the Peasant Community of Mollepatá; and to the south, with the district of Carmen Alto. According to the INEI, in the last census of 2017, the district of Ayacucho has a total population of 99,427 inhabitants, of which 97,200 live in the urban area (97.8% of the inhabitants of the district live in the study area of this research). Similarly, in 1993 the district of Ayacucho has registered a total of 18,327 private homes, for the year 2017 a total of 29,860 homes have been registered; therefore, there is an average of 2.1% annual growth rate of housing for period 1993-2017, which is reflected in the disorderly growth of housing in the city.

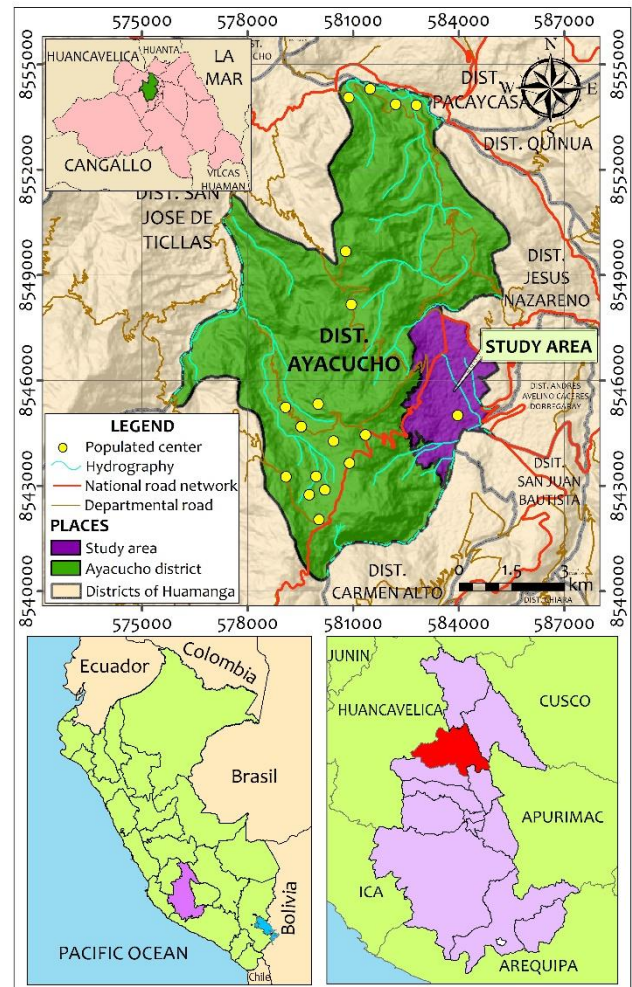


Fig. 1. Location map of the study area (Ayacucho city - Peru).

5. SEISMIC HAZARD

The use of the R-CRISIS (v20.1.0) computer program allowed the evaluation of the seismic hazard in Ayacucho city with a grid increment of 0.0025° in latitude and longitude (specifically spaced every 277 m), allowing to obtain a fairly dense data of the results. To achieve this, it was necessary to have a file with the extension shp. containing the location of Ayacucho city. Similarly, it is important to have a file extension asc. that contains the neighborhoods, associations or any other location within the study area, in which it is desired to know precisely the level of maximum acceleration of the ground. According to Kramer [20], a probabilistic seismic hazard analysis can be described as a four-step procedure: Identification and characterization of the seismogenic sources, characterization of the seismicity or temporal distribution of the seismic recurrence of each of the sources, application of the laws of attenuation to each seismogenic source, and finally the calculation of the probability of exceedance; All these steps considered in this research are described in the following sections.

5.1. SEISMICITY

The seismic activity in Peru has its origin in the friction generated by the subduction process of the Nazca plate under the South American plate and in the internal deformation of both plates (intraplate earthquakes), the latter being the ones with the greatest incidence or danger about the Ayacucho city. Silgado [21], made an important ordered description of the seismic history of Peru for the period between 1513 to 1974. From it, it has been shown that many of the earthquakes that occurred in Peru were generated outside the Ayacucho city, but due to its destructive nature with magnitudes of up to 8.4 Ms, its waves came to shake the city of Ayacucho, reaching an intensity of IV to V MM. Peru is one of the countries that has little information on the earthquakes that occurred between the 16th century and the 19th century, where the records that are available are usually not representative, since important earthquakes may have occurred in remote regions that were not recorded. It is from 1980 onwards that more information began to be obtained on a large number of seismic records that occurred in Ayacucho, reaching intensities of up to VII on the MSK scale. The areas of the districts of San José de Tillas, San Pedro de Cachi, Vinchos and Santo Tomas de Pata (very close to the district of Ayacucho) have suffered continuous and high seismic activity, due to the presence of faults and folding of several kilometers in length that have an alignment SE – NW.

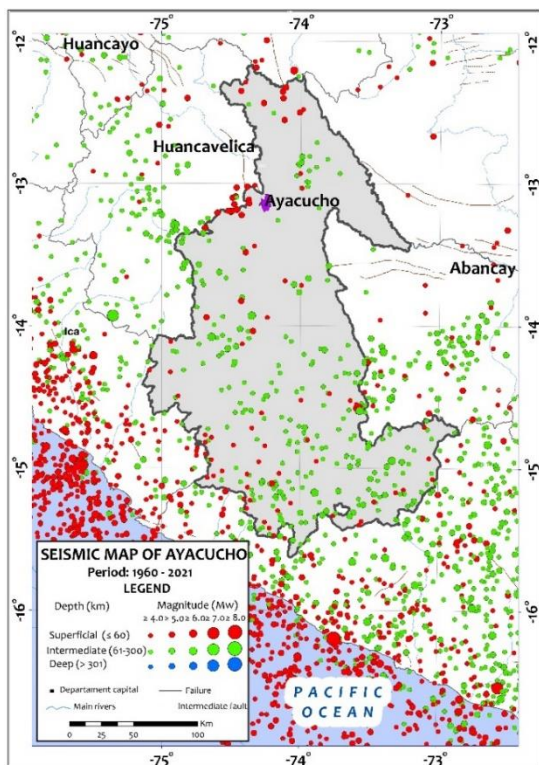


Fig. 2. Distribution map of epicenters of the department of Ayacucho.

A total of twenty-nine seismogenic sources have been used in the present research. Twenty of these were subduction, in which interface mechanisms have been differentiated (F1, F2, F3, F4, F5 and F6), superficial intraplate (F7, F8, F9, F10, F11, F12, F13, F14, F15 and F16) and intermediate (F17, F18, F19 and F20). Similarly, nine continental seismic sources have been used (F21, F22, F23, F24, F25, F26, F27, F28 and F29). Both geometries of these sources are consistent with the subduction model proposed by [22].

The geometry of each of these sources (Fig.3) was entered into the program, always keeping the Source is living option selected. Besides, the Software contemplates various types of seismic sources through the Source Type option and for the present study the area type sources were used. Together with the geometry of the source, the size of the rupture area that generates the seismic movement can be defined through the parameters K1 and K2; in the present research, for the definition of these parameters, the model of Singh et al. [23] in all seismogenic sources. In the same way, a single integral part of the source is considered with a thickness equal to zero since they are working with area-type sources.

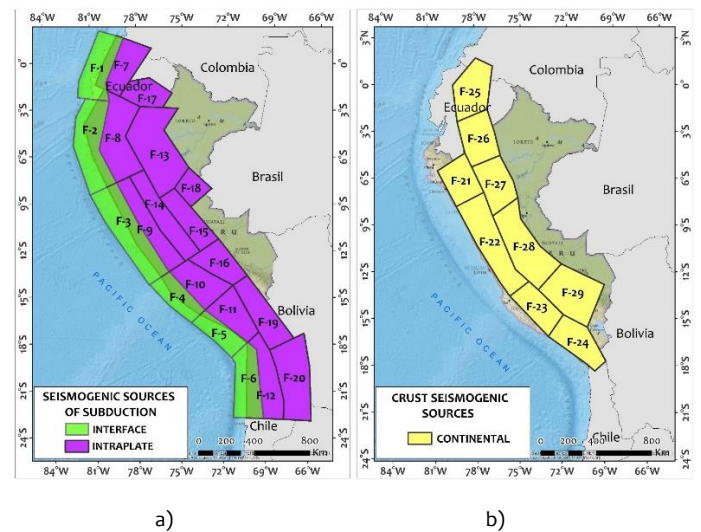


Fig. 3. Map of the seismic sources used. (a) Seismic sources of subduction (Interface and intraplate). (b) Continental seismic sources.

The seismic recurrence analysis was performed considering the Gutenberg and Richter model [24]. TABLE I shows seismological recurrence parameters of the twenty-nine seismogenic sources used. Each of these seismological parameters were integrated into R-CRISIS software, additionally, the coefficient of variation β was introduced due to the uncertainty that this parameter may have. In the same way, no standard deviation or uncertainty range of the maximum magnitude is considered (M_{max}).

TABLE I
Seismological parameters of seismogenic sources [15].

Source	seismological parameters			
	M_{min}	M_{max}	β	Rate
F1	4.3	8.1	1.286	3.1
F2	4.5	8.0	1.302	4.2
F3	4.8	8.6	1.732	3.98
F4	4.4	8.4	1.73	6.62
F5	4.8	8.4	1.473	2.58
F6	4.5	8.4	1.344	3.53
F7	4.1	7.9	0.991	1.3
F8	4.4	6.6	1.99	3.38
F9	4.5	7.3	2.04	6.47
F10	4.5	7.0	1.805	6.57
F11	4.8	7.8	2.251	4.85
F12	4.8	7.6	2.58	19
F13	4.3	7.4	1.491	3.71
F14	4.2	6.6	1.28	2.07
F15	4.6	6.0	2.334	2.52
F16	4.1	6.9	1.307	1.25
F17	4.4	7.3	1.697	3.87
F18	4.8	7.1	2.413	3.13
F19	4.8	7.0	2.307	2.92
F20	4.5	7.7	2.352	11.7
F21	4.0	7.1	0.58	0.461
F22	4.1	7.2	1.068	1.32
F23	4.5	7.0	1.384	0.675
F24	4.4	7.4	1.314	1.28
F25	4.2	7.0	1.133	2.04
F26	4.6	6.7	1.384	0.889
F27	4.8	6.8	2.162	1.27
F28	4.5	7.0	1.667	2.85
F29	4.3	7.0	1.259	1.42

In order to determine the seismic response of the ground in a given place produced by a seismic event in a specific source, the attenuation laws are used. In Peru, several attenuation laws have been used, prepared with seismic records from Peru and other countries; a large number of these were analyzed by [25] who concluded that, among all the attenuation laws analyzed, they are those of Youngs et al. [26] and Sadigh et al. [27] which yielded average results of acceleration values and that these are consistent with tectonics and seismotectonics in Peru. Therefore, in order to determine the seismic response in Ayacucho city, the attenuation laws of Youngs et al. for subduction earthquakes and those of Sadigh et al. for continental earthquakes are used.

5.2. SEISMIC MICROZONATION

The study area at the regional level corresponds to the geomorphological unit called Penillanura Disectada. On the other hand, at the local level, the geomorphology of the Ayacucho city is made up of units of ravines and slopes which have given rise to a cover of alluvial deposits that are overlying the sedimentary deposits of the Ayacucho formation. The local geology of Ayacucho city is composed of rocks of sedimentary and volcanic origin whose ages range between the upper Tertiary and the recent Quaternary. Specifically, in study area, the sedimentary deposits of the Ayacucho formation and DOI: <https://doi.org/10.21754/tecnica.v32i2.1377>

the alluvial deposits are found in a greater proportion, and in a lesser proportion there are volcanic deposits of the Molinoyoc formation that overlie deposits of the Ayacucho formation.

In order to characterize the soil in Ayacucho city, a large database of soil studies prepared by [28] and [29] was taken into account. It was possible to collect information on classification, exploration and sampling of soils and rocks, as well as values of soil bearing capacities and some seismic refraction tests. In order to achieve a seismic geotechnical zoning in Ayacucho city, geophysical tests MASW (Multichannel Analysis of Surface Waves) were carried out and allowed to soil characterize from shear wave velocities. Fig. 4 depicts a map of the distribution of seismic refraction probing, as part of the information collected, as well as the MASW probing of the present study.

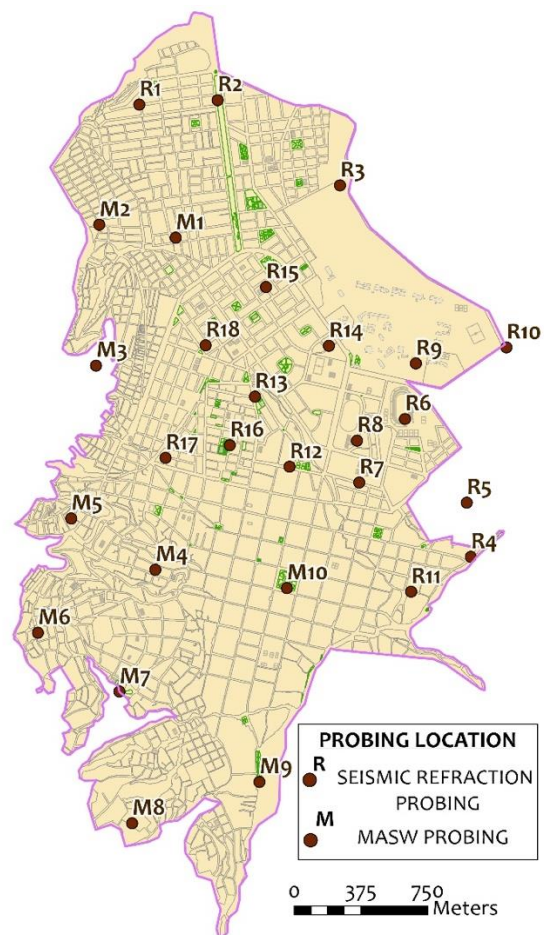


Fig.4. Location map of seismic refraction and MASW probing in the study area.

“The MASW method or Multichannel Analysis of Surface Waves allows to determine the stratigraphy of the subsoil under a point indirectly, based on the

change in the dynamic properties of the materials that make it up” [30, p. 35]. Each of these tests were carried out at various points in study area, distributed in such a way that the results could be interpolated in order to generate seismic zoning maps. In addition, to achieve this end, the optimal field parameters of a MASW survey established by [31] were taken into account.

Once the field work was completed, data processing continued. This process consisted of obtaining dispersion curves (a curve that relates phase velocity versus frequency) (Fig. 6) from the seismic wave record obtained in each field scan line (Fig. 5). Then, through an iterative process (inversion method), shear wave velocity profiles (V_s) were obtained using the different modules of the SeisImager/SW program (Fig. 7).

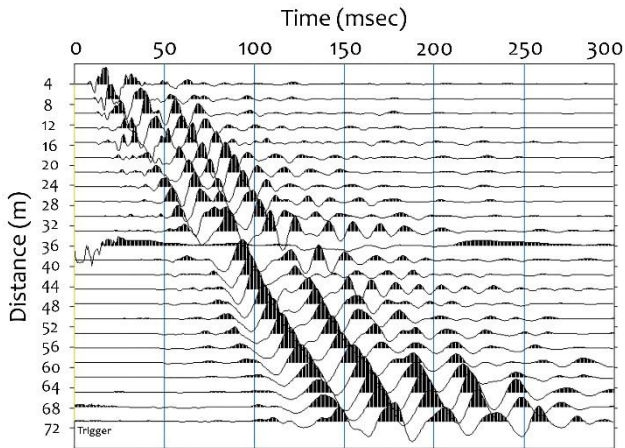


Fig. 5. Seismic waves recorded in the MASW test.

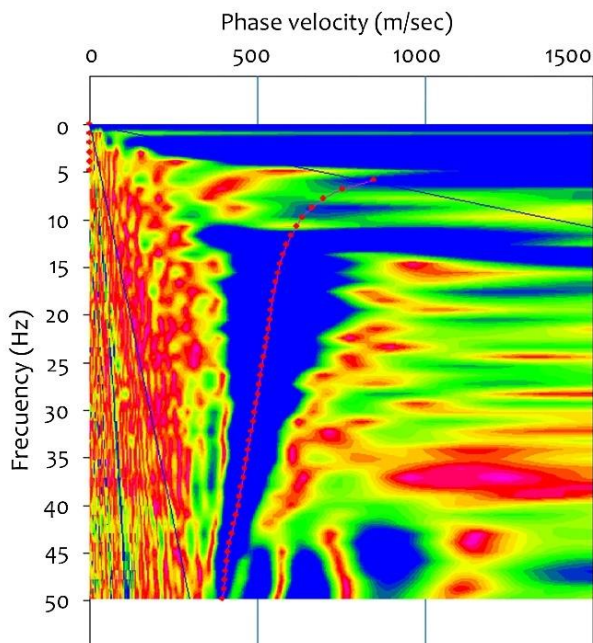


Fig. 6. Dispersion curve.

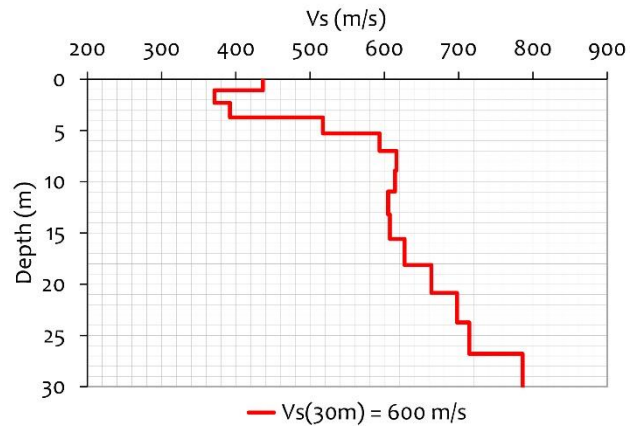


Fig. 7. Shear wave velocity profile (V_s) in MASW test.

The soil, geological, geotechnical and geophysical studies have made possible to describe the mechanical soil characteristics and its seismic response. With these results, a seismic geotechnical zoning map of Ayacucho city is obtained, as can be shown in Fig. 8. This map presents 3 zones, where the first two are subdivided into A and B. Zone IA differs from zone IB by the type of material and by bearing soil capacity. Zone IIA differs from zone IIB due to the bearing soil capacity and the land slope, where zone IIA presents lower soil resistance and greater slopes. A better description of each zone is shown in TABLE II. This table shows values such as the bearing capacity of the soil obtained from the Meyerhof formula, for which the geomechanical properties of the geological units and direct shear tests were taken into account as part of the information collected. On the other hand, the seismic amplification of the soil obtained from the one-dimensional propagation theory of shear waves in soil deposits is also described, using three recorded input earthquakes as excitation force through the SHAKE program.

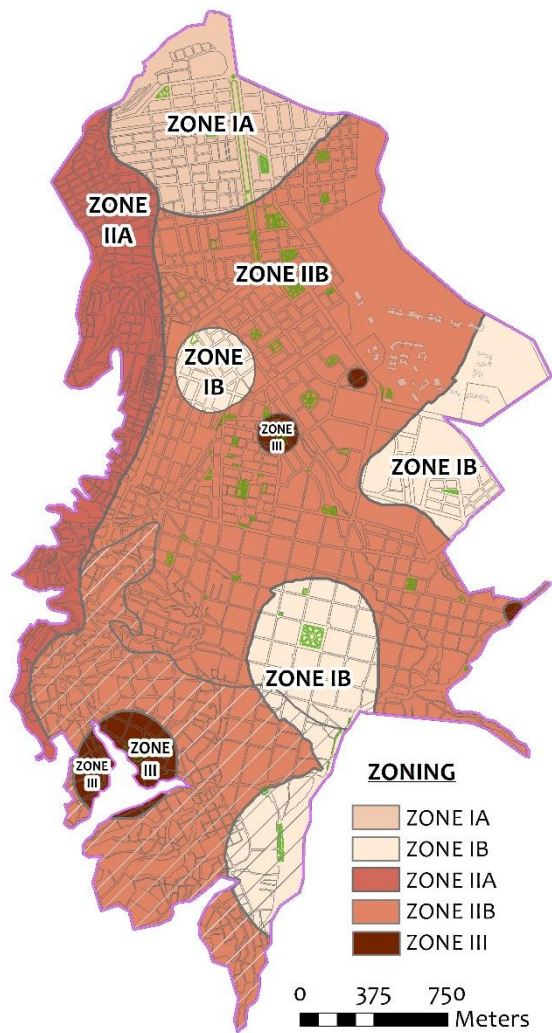


Fig. 8. Seismic geotechnical zoning map for Ayacucho city.

TABLE II
Description of geotechnical and seismic features of prevalent soil types in Ayacucho city.

Zones	Color	Description
ZONE IA		Inorganic silt of low to high plasticity, very consolidated, lacustrine origin, firm consistency, located on flat terrain or with a slight slope (0° to 10°); bearing capacity of foundation soil between 1.0 to 1.5 kg/cm ² , seismic amplification of 1.0 to 1.5.
ZONE IB		Silt sandy gravel, formed by sub-rounded stones of sedimentary origin mixed in a silty-sandy material with low plasticity. In the Alameda there is silty sand material, formed by the alteration of tobaceous sandstones with combinations of "caliche volcanic".
ZONE IIA		Both are located on slight slope terrain (0° to 15°), medium to dense state of compaction; bearing capacity of foundation soil between 1.5 to 2.5 kg/cm ² , seismic amplification of 1.0 to 1.5.
ZONE IIB		Well-graded silty-sandy gravel of sedimentary origin (Pleistocene conglomerates), medium to dense state of compaction, located on steep slopes (30° to 60°) susceptible to water erosion; bearing capacity of foundation soil between 1.0 to 1.5
ZONE III		

kg/cm², seismic amplification of 1.5 to 2.0 with a tendency to increase these amplification factors due to topographic effects and possible slope instability problems.

ZONE IIB Silty Gravel
Silty sandy gravel of lake and alluvial origin, medium to dense state of compaction and silty sand of low plasticity with semi-compact to compact state; both materials located on medium to steep slopes (10° to 45°), susceptible to water erosion; bearing capacity of foundation soil between 1.5 to 2.0 kg/cm², seismic amplification of 1.5 to 2.0.

ZONE III Silty Sand
Silt sandy gravel of low plasticity and sedimentary origin with a slight slope (0° to 10°), located in two areas in the center of Ayacucho city. On the other hand, the areas in the southern part of this city present silty sand in a semi-compact state with steep slopes (30° to 60°) susceptible to water erosion. In both cases, the bearing capacity of foundation soil varies between 1.0 to 2.0 kg/cm² and presents a seismic amplification of 2.0 to 2.5.

5-3. EVALUATION OF THE SEISMIC HAZARD

The seismic hazard curves are determined under the probability theorem: the probability that the intensity *I* is greater than or equal to a specific value *i*, is calculated for a possible earthquake at a possible location within the source and then multiplied by the probability that specific magnitude can occur at that location. Then, the process is repeated for all possible magnitudes and locations, to finally add all of them. The calculation of the seismic hazard for a specific place of interest is influenced by the magnitude (*m*) and the distance (*r*), being $P(I \geq i)$ defined by equation (1).

$$P(I \geq i) = \iint P[I \geq i / (m,r)] f_m(m) f_r(r) dm dr \quad (1)$$

Where $P[I \geq i / (m,r)]$ is the probability that the intensity *I* exceeds a specific value *i* for a possible earthquake located within the source, and it is obtained through the attenuation relationship. Also, $f_m(m)$ and $f_r(r)$ are the probability density functions for magnitude and distance.

Equation (1) summarizes the theory developed by Cornell [32] to analyze seismic hazard. The evaluation of this integral is given by the R-CRISIS software, which was developed by Ordaz et al. [33] in calculating the seismic hazard.

As part of the results of the probabilistic evaluation of seismic hazard, the R-CRISIS program allowed knowing the maximum acceleration of the ground at any point within the study area. For a better representation, in Fig. 9 a map of isoaccelerations of Ayacucho city is presented for a

return period of 475 years (design earthquake, according to the E.030 Peruvian Norm). In addition, in TABLE III, the results of the maximum ground accelerations for the different seismic scenarios analyzed are presented.

TABLE III
Peak ground acceleration (PGA) at bedrock level expected in Ayacucho city.

Seismic scenario	Return period	PGA
Frequent earthquake	43 years	0.3064
Occasional earthquake	72 years	0.3068
Rare earthquake	475 years	0.3103
Very rare earthquake	970 years	0.3103

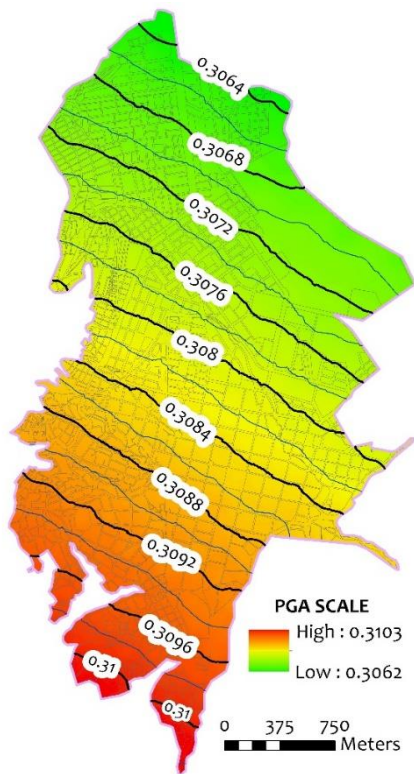


Fig. 9. Isoacceleration map for Ayacucho city (Tr: 475 years).

Another of the results obtained from the use of the R-CRISIS program are the seismic hazard curves, also known as hazard curves, which represent the

interrelation between a range of ground motion levels and the probability of exceedance for a given period determined exposure. These seismic hazard curves will be very useful in the seismic risk assessment process when the damage level of buildings is knowing for any other seismic scenario that has not been described in TABLE III. Furthermore, the hazard curves are presented in Fig. 10 for each structural period considering an exposure period of 50 years.

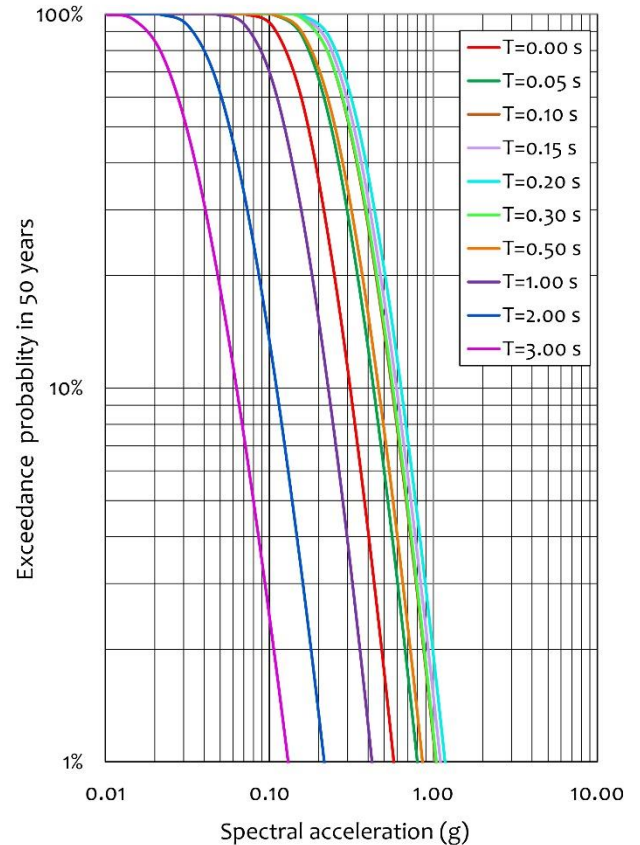


Fig. 10. Seismic hazard curves for Ayacucho city.

In the same way as the seismic hazard curves, the uniform hazard spectra were obtained, which help the interpretation of the seismic hazard as well as the construction of design spectra. The results of the uniform hazard spectra for each seismic scenario analyzed in this research are shown in Fig. 11. Besides, a spectrum of its own design for Ayacucho city according to Standard Code E.030 can be obtained from the uniform red hazard spectrum (Fig. 11), whose spectral values have the same probability of exceedance (10%) in all structural periods for an exposure period of 50 years.

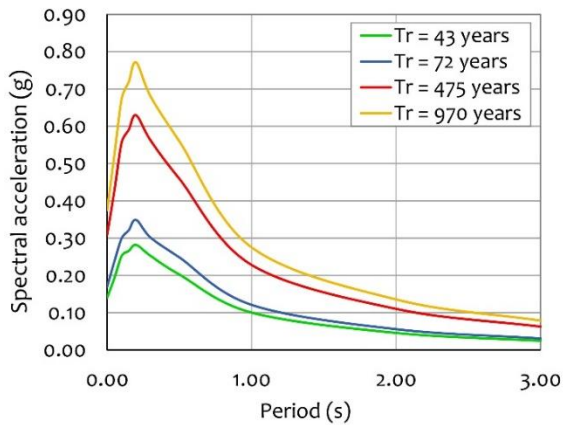


Fig. 11. Uniform hazard spectra for each seismic scenario.

6. SEISMIC VULNERABILITY

The vulnerability index methodology was used to estimate the buildings seismic vulnerability [34], which consists in analysis a total of eleven parameters (Fig. 12) that characterize the susceptibility of a structure to suffer some type of damage after the occurrence of an earthquake. These parameters are:

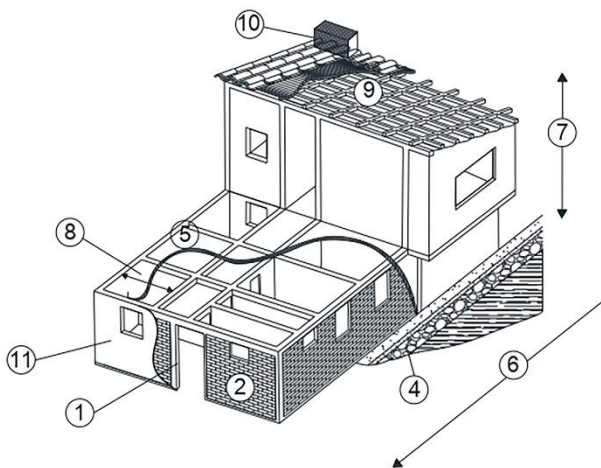


Fig. 12. Seismic vulnerability model parameters [17].

- (1) Organization of the resilient system.
- (2) Quality of resistant system.
- (3) Conventional resistance.
- (4) Building position and foundation.
- (5) Horizontal diaphragm.
- (6) Plant configuration.
- (7) Elevation configuration.
- (8) Maximum separation between walls or columns.
- (9) Type of cover.
- (10) Non-structural elements.
- (11) State of conservation.

The vulnerability index method was developed for two large groups of structures: Masonry and reinforced concrete, where the greatest number of studies and application of this methodology were in masonry structures, due to the large number of these

in various countries. However, in the case of reinforced concrete structures, software and numerical methods are generally used that require greater computational cost.

6.1. VULNERABILITY INDEX

The vulnerability index for confined and adobe masonry structures is calculated as the weighted sum of the numerical values of the factor K_i by the weight factor W_i , of each of the parameters that are considered to characterize the seismic behavior of the structure, as shown in equation (2).

$$I_v = 100 * \frac{\left(\sum_{i=1}^{11} K_i * W_i \right)}{382.5} \tag{2}$$

During field data collection, each parameter is assigned to the four A, B, C, D classes, depending on the condition of analyzed parameter, which ranges from A (Optimal) to D (Unfavorable). Where each of these classes corresponds to a numerical value K_i that varies between 0 and 45, this can be seen in TABLE IV. In addition, it can be seen that each parameter is affected by a weight factor W_i , which, according to [35, p. 39], reflects the importance of each of the parameters within the resistant system of the structure.

TABLE IV
Values of K_i and weight (W_i) for masonry buildings. [34].

i	Parameter	K_i				W
		A	B	C	D	
1	Resistant system organization	0	5	2	4	1.0
2	Resistant system quality	0	5	2	4	0.5
3	Conventional resistance	0	5	2	4	1.5
4	Building position and foundation	0	5	2	4	0.5
5	Horizontal diaphragms	0	5	1	4	1.0
6	Plant configuration	0	5	2	4	0.5
7	Elevation configuration	0	5	2	4	1.0
8	Maximum distance between walls	0	5	2	4	0.5
9	Cover type	0	1	2	4	1.0
10	Non-structural elements	0	0	2	4	0.5
11	Conservation state	0	5	2	4	1.0

The evaluation of the vulnerability index for reinforced concrete structures is carried out in a similar way for masonry structures, but in this case only three ratings are assigned (A, B, C), where each assigned class corresponds to a numerical value K_i that varies between -1 and 3, it can be seen in TABLE V. Finally, the vulnerability index is obtained from a weighted sum of the weight factors W_i and K_i considering equation (3).

$$I_v = 100 * \frac{\left(\sum_{i=1}^{11} K_i * W_i \right) + 1}{34} \tag{3}$$

TABLE V
Values of K_i and weight (W_i) for reinforced concrete buildings [34].

i	Parameter	K_i			W_i
		A	B	C	
1	Resistant system organization	0	1	2	4.0
2	Resistant system quality	0	1	2	1.0
3	Conventional resistance	-1	0	1	1.0
4	Building position and foundation	0	1	2	1.0
5	Horizontal diaphragms	0	1	2	1.0
6	Plant configuration	0	1	2	1.0
7	Elevation configuration	0	1	3	2.0
8	connection between critical elements	0	1	2	1.0
9	Cover type	0	1	2	1.0
10	Nonstructural elements	0	1	2	1.0
11	Conservation state	0	1	2	1.0

Finally, the level of vulnerability for both masonry and reinforced concrete is classified according to TABLE VI.

TABLE VI
Vulnerability level as a function of the vulnerability index [12].

I_v range	Vulnerability level
$0\% \leq I_v < 20\%$	Low
$20\% \leq I_v < 40\%$	Medium
$40\% \leq I_v \leq 100\%$	High

6.2. DATA COLLECTION

In the Ayacucho city it is common to see confined masonry buildings with poor structuring, adobe buildings in poor condition, very old colonial buildings and among many other conditions that show their precariousness, as can be seen in Fig. 13. In this sense, in order to characterize the degree of seismic vulnerability of the building, previously, the data collection of the eleven parameters that encompasses the methodology proposed by [34] is carried out, after that, the vulnerability index is calculated from the use of expressions (2) and (3). To achieve this goal, it was necessary to label each block in a cadastral map with a respective number that allows data to be collected in an orderly manner.

Then, the data collection of eleven parameters is carried out using a field evaluation form that allows collecting the information to characterize the vulnerability index. This evaluation form was previously validated by the opinion of experts, who previously carried out research work related to seismic vulnerability and seismic risk. The use of this instrument requires prior knowledge for a correct assignment of classes A, B, C and D that encompasses each of eleven proposed parameters. The evaluation criteria were considered taking into account the recommendations given by [36] and [37] for masonry buildings, and by [12] for reinforced concrete buildings.



Fig. 13. Typical buildings that present a certain seismic vulnerability level in Ayacucho city.

The data of the most representative building in each block was collected taking into account the structural system and the type of material to be evaluated. For each block, three representative buildings were evaluated: one made of adobe, one of confined masonry, and one of reinforced concrete. It was carried out in order to take into account all the types of buildings present in Ayacucho city and that are better classified.

All the information collected in study area was emptied into a database taking into account each structural typology. Likewise, the vulnerability index was calculated in an automated manner and this entire database was stored in a Geographic Information System (ArcGIS) to generate the seismic vulnerability maps.

6.3. EVALUATION OF THE SEISMIC VULNERABILITY

Once the vulnerability index of the buildings had been evaluated, the degree of vulnerability was classified according to structural typology (TABLE VII). These results are shown graphically in Fig. 14, where 80.98% of the adobe buildings have medium vulnerability, being the most prevalent level of

vulnerability in the study area. In addition, buildings with high and low vulnerability represent 9.77% and 9.25% respectively. In the case of confined masonry buildings, there is a slight advantage of buildings with medium vulnerability (53.53%) over those with low vulnerability (44.90%) and a minimum percentage of buildings with high vulnerability (1.58%). On the other hand, 100% of the reinforced concrete buildings showed low vulnerability. The general results of TABLE VII show that of the 2078 buildings evaluated: 29.40% have a low vulnerability; 65.26%, medium vulnerability; and 5.34%, high vulnerability.

TABLE VII
Results of the seismic vulnerability level in buildings evaluated.

Vulnerability level	Adobe masonry		Confined masonry		Reinforced concrete		Total	
	N	%	N	%	N	%	N	%
Low	8	9.25%	4	44.90%	3	10.00%	15	29.40%
	9	9.77%	4	40.00%	8	26.67%	21	40.00%
Medium	7	7.98%	5	53.53%	0	0.00%	12	22.53%
	7	7.98%	7	73.33%	0	0.00%	14	26.53%
	9	9.77%	7	73.33%	0	0.00%	16	30.00%
High	9	9.77%	1	10.42%	0	0.00%	10	18.77%
	4	4.25%	7	73.33%	0	0.00%	11	20.77%
Total	9	10.00%	10	10.42%	3	3.33%	22	40.00%
	6	6.00%	7	7.29%	8	8.00%	21	38.71%

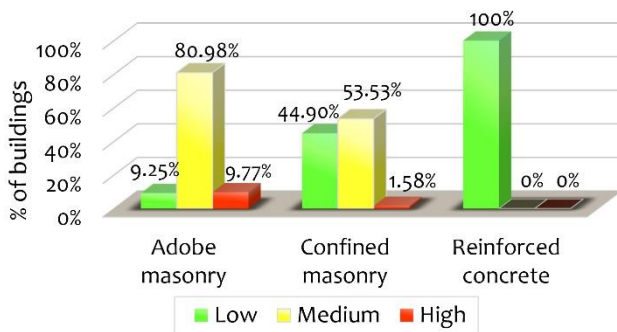


Fig. 14. Results of the seismic vulnerability level in buildings evaluated.

Additionally, for a better graphical representation of the results, a seismic vulnerability map was obtained at the block level, as shown in Fig. 15. The preponderance of the average vulnerability of the buildings is represented with the color yellow. The blank spaces correspond to markets whose structural system was not analyzed, likewise the cream and light green spaces represent stadiums and parks respectively, where a level of vulnerability was not evaluated due to the fact that there are no buildings.

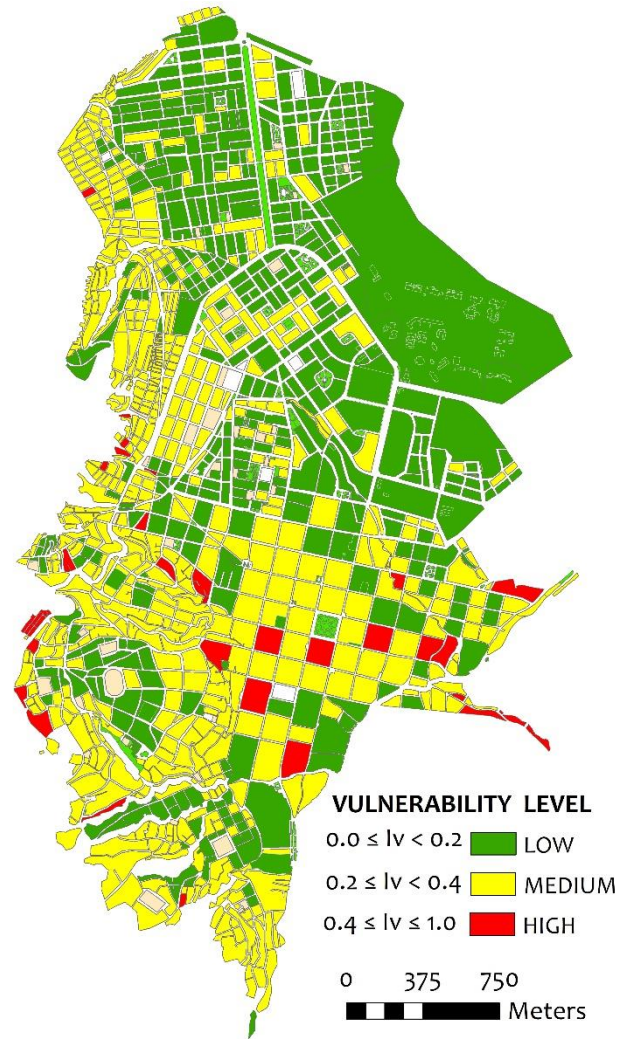


Fig. 15. Seismic vulnerability map for Ayacucho city.

From Fig. 15, it can be seen that in the central area of Ayacucho city, located around Sucre Park, the buildings have a high and medium vulnerability level for the most part, this as a consequence of the large number of colonial buildings in poor condition and with adobe materials that have cracks due to some seismic movements produced in the city. Likewise, it can be noted that in the peripheral areas located to the west of the city, the buildings also present a high and mostly medium level of vulnerability, and this as a consequence of the disorderly growth of the city whose buildings are self-constructed on land with steep slopes without no slope stability analysis.

7. SEISMIC RISK

7.1. DAMAGE INDEX

There are a variety of methodologies for assessing seismic risk. For instance, it is possible to estimate the level of damage as a function of the maximum structural distortion, where the latter is estimated using the methodology presented in [38] and [39], supported by [40]. In the present research, the

damage level of buildings is estimated using the vulnerability functions, which relate the normalized vulnerability index (I_v) with the damage index (I_D) conditioned by the maximum soil acceleration. The vulnerability functions proposed by Grimaz [41] are used here for adobe buildings (Fig. 16); and those of Quispe [12], for confined masonry and reinforced concrete buildings (Fig. 17).

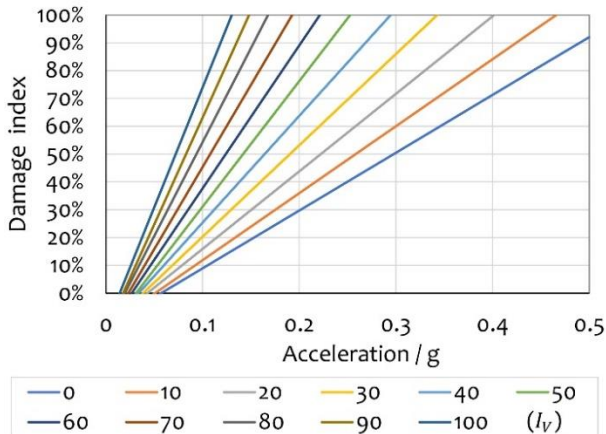


Fig. 16. Vulnerability functions for adobe buildings [41].

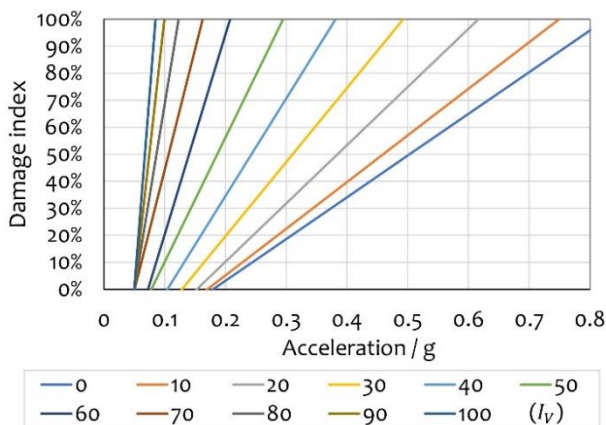


Fig. 17. Vulnerability functions for confined masonry buildings [12].

Once the damage index of each building has been determined, it is necessary to characterize the level of damage to them, for this, TABLE VIII, recommended by the Earthquake Engineering Research Institute of California was used. In this case, the damage index data was obtained in the previous step as a result of vulnerability functions, besides, the structural damage level is considered as: None, Light, moderate, considerable, strong, severe or collapse.

Likewise, a brief definition of structural damage and non-structural elements that each level of damage implies is made, including the repairing structural cost. This allows giving a better idea of the consequences that an eventual earthquake of a certain magnitude can bring.

TABLE VIII
Description of scales and damage levels [17].

Damage level	Damage index (%)	Definition
None	0 – 2.5	The damage to structural elements is negligible with small cracks in non-structural elements. No repair costs are required and the effect on structural behavior is negligible.
Slight	2.5 – 7.5	The damage to non-structural elements is widespread and causes tile detachment. Possible presence of cracks in structural elements such as load-bearing walls, beams or columns, where a small percentage of the structure requires repair. The effect on structural behavior is minimal.
Moderate	7.5 – 15	The damage to non-structural elements is very important and the cost of repair increases. Damage to structural elements can be significant, affecting the structural behavior to a lesser percentage, however, the stability of the structure is not compromised.
Considerable	15 - 30	The damage to the structural elements is significant and requires extensive repairs, although it is not necessary to leave the building. The repair cost can be a significant percentage of the cost of the structure.
Strong	30 – 60	The damage to the structural elements is very important and the risk of structural stability is high. In most cases, the repair cost can be a very important percentage of the cost of the structure and it is necessary to leave the building for people safety.
Severe	60 – 90	The damage to the structural elements is very considerable and the stability of the structure is dangerous. The cost of repair is equal to or greater than the cost of the building and demolition is recommended. In addition, evacuation of the building is required.
Collapse	90 - 100	Building declared in ruins, total collapse.

7.2. EVALUATION OF SEISMIC RISK IN AYACUCHO CITY

In this section, the level of damage is evaluated for four seismic scenarios: frequent, occasional, rare and very rare earthquakes. TABLE IX shows the results of the quantity and percentage of buildings that suffer a certain damage level, depending on the intensity of earthquake. Likewise, the statistical results are presented in Fig. 18.

The results show that when a frequent earthquake is presented (0.14g), it is estimated that 39.7% of total buildings would not suffer any damage;

10.5% of buildings would suffer slight damage, affecting non-structural elements in a general way; 3.0% of buildings would suffer moderate damage, causing significant damage to non-structural elements and affecting structural behavior to a lesser extent. The 11.4% of buildings would suffer considerable damage, where the structural elements would suffer significant damage requiring major repairs; 35.3% of the buildings would suffer a strong damage that implies that the structural elements have very important damages, putting the stability of the structure at risk and where the cost of repair can represent a high percentage of structural cost. Finally, the 0.1% of buildings would suffer severe damage, causing the stability of the structure to be precarious due to great damage in structural elements.

Analogously to the previous considerations, for an occasional earthquake (0.17g), it is estimated that 20.8% of buildings would not suffer any damage; 11.5% would suffer light damage; 15.9%, moderate damage; 5.2%, considerable damage; 45.2%, strong damage; 1.3%, severe damage; and the rest of the buildings would collapse.

Similarly, for a rare or design earthquake (0.31g), it is estimated that 20.1% of the buildings would suffer considerable damage; 30.0%, strong damage; 31.0%, severe damage; and the rest may collapse.

For a very rare earthquake (0.37g), it is estimated that 0.1% of the buildings would suffer considerable damage; 36.1%, strong damage; 18.4%, severe damage; and the rest may collapse.

On the other hand, the highest percentage of buildings would have a strong or severe damage for a design earthquake (0.31g). This can compromise the stability of the structure. In this way, the great risk posed by most of the buildings in Ayacucho city is shown, which would be heavily damaged after an earthquake whose return period is 475 years. In addition, if it is a very rare earthquake, whose return period is 970 years, almost all the buildings would have a damage greater than the strong one, which implies that most of them would be unusable or uninhabitable.

TABLE IX

Results of damage level of buildings evaluated for each seismic scenario.

Damage level	0.14g		0.17g		0.31g		0.37g	
	N°	%	N°	%	N°	%	N°	%
None	824	39.7%	433	20.8%	0	0.0%	0	0.0%
Slight	218	10.5%	239	11.5%	0	0.0%	0	0.0%
Moderate	62	3.0%	331	15.9%	0	0.0%	0	0.00%
Considerable	238	11.4%	107	5.2%	417	20.1%	1	0.1%

Strong	734	35.3%	940	45.2%	623	30.0%	750	36.1%
Severe	2	0.1%	27	1.3%	645	31.0%	383	18.4%
Collapse	0	0.0%	1	0.1%	393	18.9%	944	45.4%

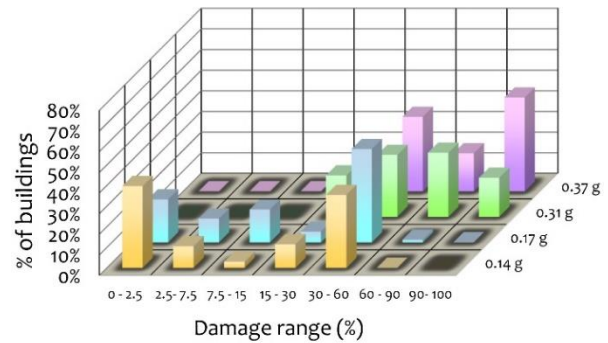


Fig. 18. Percentage of buildings that suffer a damage level for each seismic scenario.

Finally, Fig. 19 plots the seismic risk maps for different seismic scenarios analyzed. From which, it can be seen that the degree of damage is greater each time the seismic intensity (PGA) increases.

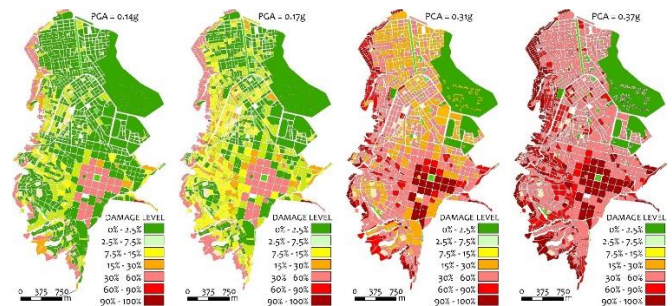


Fig. 19. Damage level measured for different peak ground accelerations (PGA) caused by each seismic scenario.

From Fig. 19, in accordance with the results of seismic vulnerability, it can be seen that the buildings located in the downtown area of the city are the ones that would suffer the greatest damage due to a seismic movement, these results are more noticeable for the scenarios seismic rare (PGA=0.31g) and very rare (PGA=0.37g) in which it is clearly distinguished that, like the central part of the city, the buildings located in the peripheral areas of urban expansion present the highest levels of damage. From the above precision it can be concluded that the buildings of Ayacucho city with a medium vulnerability level are exposed to strong and severe damage after the occurrence of a rare earthquake and even collapse for a very rare earthquake; Similarly, buildings with a high level of vulnerability are very likely to collapse after the occurrence of an earthquake whose maximum ground acceleration exceeds 0.31g.

CONCLUSIONS

- The city of Ayacucho has buildings that are very vulnerable to damage in the event of a seismic

event. There is a prevalence of 65.26% of buildings with medium vulnerability, followed by 29.40% of buildings with low vulnerability and only 5.34% with high vulnerability. The main parameters that conditioned these results were the configuration in plan and elevation, since it is common to observe buildings with poor structuring in the city. Likewise, the city of Ayacucho is located on steep slopes, a factor that conditioned many of the buildings to present the problem of soft floors, since in order to overcome the large slopes, the construction of column and beam porticos became common in the first levels and only in the upper levels the confined masonry.

- The maximum soil acceleration in Ayacucho city for a design earthquake is 0.31g, which diverges from the value $Z = 0.25$ (Zone 2) established by the Peruvian Standard E.030 for Seismic Resistant Design. In addition, taking into account that the evaluated areas include urban and peripheral zones of urban expansion, the results obtained in this research show that the Ayacucho city is located between zone 2 ($Z = 0.25g$) and zone 3 ($Z = 0.35g$).
- Most of the Ayacucho city is made up of silty gravel (GM) and silty sand (SM); with a small part, north of the city, made up of low to high plasticity inorganic slime. Geologically there are recent alluvial deposits overlying the sedimentary deposits of Ayacucho formation, with a state of loose compaction on slopes and compact in flat areas. The bearing capacity of soils varies from 1.0 to 2.5 kg/cm^2 and presents a seismic amplification of 1.0 to 2.0, with certain small areas where the seismic amplification reaches 2.5.

REFERENCES

- [1] S. Kwag, A. Gupta, J. Baugh, and H. Kim, "Significance of multi-hazard risk in design of buildings under earthquake and wind loads", *Engineering Structures*, vol. 243, 2021.
- [2] A. Dunant, M. Bebbington, and T. Davies, "Probabilistic cascading multi-hazard risk assessment methodology using graph theory, a New Zealand trial", *International Journal of Disaster Risk Reduction*, vol. 54, 2020.
- [3] S. Kameshwar and J. Padgett, "Multi-hazard risk assessment of highway bridges subjected to earthquake and hurricane hazards", *Engineering Structures*, vol. 78, pp. 154-166, 2014.
- [4] Y. Liu et al., "Scenario-based seismic vulnerability and hazard analyses to help direct disaster risk reduction in rural Weinan, China", *International Journal of Disaster Risk Reduction*, vol. 48, 2020.
- [5] J. Ramos, "Microzonificación sísmica del distrito de Ancón", tesis de grado, Univ. Nac. Ingeniería, Lima, Perú, 2017 [Online]. Available: <http://cybertesis.uni.edu.pe/handle/uni/9524>
- [6] A. Pretell y Z. Aguilar, "Microzonificación geotécnica sísmica para el ámbito Pisco, San Clemente, Túpac Amaru, San Andrés y Paracas", 2014 [Online]. Available: <https://app.ingemmet.gob.pe/biblioteca/pdf/CPG16-038.pdf>

- The preponderance of average buildings vulnerability of study area conditioned that seismic risk studies show that there is a prevalence of 61% in buildings that would suffer strong or severe damage after the occurrence of a rare or design earthquake ($T_r = 475$ years). Also, some buildings could collapse. The buildings located in the peripheral areas and those in historic center are would suffer the most damage, this due to self-construction and the colonial buildings in deplorable states of conservation. Depending on the structural typology, confined masonry buildings could suffer great damage. In addition, adobe buildings would be the most damaged because they would have severe damage and 40.6% could collapse. As a final conclusion, these results are alarming and serve for both the government, community organizations and the population to take human actions, political actions and strategies at the government level that allow disaster mitigation and resilience.

ACKNOWLEDGEMENTS

The authors would like to gratefully acknowledge to the National University of San Cristobal de Huamanga (UNSCH), for financing this research after winning the second thesis grant contest with FOCAM resources. The authors would also like to acknowledge to Provincial Municipality of Huamanga, the Office of Territorial Organization and Sub-Management of Civil Defense of the Regional Government of Ayacucho for the information utilized in this research work.

- [7] R. Delgadillo, "Microzonificación geotécnica sísmica del distrito de Independencia - Lima", thesis, Univ. Nac. San Cristóbal de Huamanga, Ayacucho, Perú, 2014 [Online]. Available: <http://repositorio.unsch.edu.pe/handle/UNSCH/784>
- [8] U. Mena, "Evaluación del riesgo sísmico en zonas urbanas", doctoral thesis, Univ. Pol. Catalunya, 2002 [Online]. Available: <http://hdl.handle.net/2117/93534>
- [9] P. Angeletti, A. Bellina, E. Guagenti, A. Moretti, and V. Petrini, "Comparison between vulnerability assessment and damage index, some results", in the 9th World Conference on Earthquake Engineering, Tokyo-Kyoto, 1988, pp. 181-186.
- [10] A. Barbat, F. Yépez, and J. Canas, "Damage scenarios simulation for seismic risk assessment in urban zones", *Earthquake spectra*, vol. 12, no. 3, pp. 371-394, 1996.
- [11] E. Maldonado, I. Gómez, and G. Chio, "Simulating vulnerability functions and seismic damage probability matrix for reinforced concrete frame buildings", *Ingeniería e Investigación*, vol. 18, no. 3, pp. 28-40, 2008.
- [12] N. Quispe, "Evaluación del riesgo sísmico en la ciudad de Ayacucho", master's thesis, Univ. Nac. Ingeniería, Lima, Perú, 2004 [Online]. Available: <http://cybertesis.uni.edu.pe/handle/uni/2876>

- [13] J. Castillo y J. Alva, "Peligro sísmico en el Perú", VII Congreso Nacional de Mecánica de Suelos e Ingeniería de Cimentaciones, Lima, 1993.
- [14] A. Bolaños y O. Monroy, "Espectros de peligro sísmico uniforme", master's thesis, Pont. Univ. Católica del Perú, 2004 [En línea]. Disponible en: <http://hdl.handle.net/20.500.12404/1331>
- [15] M. Roncal, "Determinación del peligro sísmico en el territorio nacional y elaboración de aplicativo web", thesis, Univ. Nac. Ingeniería, Lima, Perú, 2017 [Online]. Available: <http://cybertesis.uni.edu.pe/handle/uni/12214>
- [16] F. Marín, "Evaluación del riesgo sísmico del centro histórico de Huánuco", master's thesis, Univ. Nac. Ingeniería, Lima, Perú, 2012 [Online]. Available: <http://cybertesis.uni.edu.pe/handle/uni/1236>
- [17] M. Silva y C. Quesquén, "Estudio de riesgo sísmico, en la ciudad de Chiclayo, zona este (av. Sáenz Peña, av. Castañeda Iparraguirre, av. Nicolás de Piérola, av. Jorge Chávez, y av. Bolognesi)", thesis Univ. Nac. Pedro Ruiz Gallo, Lambayeque, Perú, 2020 [Online]. Available: <https://hdl.handle.net/20.500.12893/8115>
- [18] J. Olarte, J. Julca, y E. Orbegoso, "Evaluación del riesgo sísmico del centro histórico de Chiclayo", Univ. Nac. Pedro Ruiz Gallo, Lambayeque, Perú, 2008 [Online]. Available: <https://bit.ly/3uRTBeo>
- [19] A. Castillo, "Seismic risk scenarios for buildings in Mérida, Venezuela. Detailed vulnerability assessment for non-engineered housing", doctoral thesis, Univ. Pol. Catalunya, 2005 [Online]. Available: <http://hdl.handle.net/2117/93848>
- [20] S. Kramer, *Geotechnical earthquake engineering*. New Jersey, USA: Pearson Prentice Hall, 1996.
- [21] E. Silgado, "Historia de los Sismos más Notables Ocurridos en el Perú (1513-1974)", *Instituto de Geología y Minería, Serie C: Geodinámica e Ingeniería Geológica*, Boletín no. 3, 1978.
- [22] T. Cahill and B. Isacks, "Seismicity and shape of the subducted Nazca plate", *Journal of Geophysical Research: Solid Earth*, vol. 97, no. B12, pp. 17503-17529, 1992.
- [23] S. Singh, E. Bazan, and L. Esteve, "Expected earthquake magnitude from a fault", *Bulletin of the Seismological Society of America*, vol. 70, no. 3, pp. 903-914, 1980.
- [24] B. Gutenberg and C. Richter, "Frequency of earthquakes in California", *Bulletin of the Seismological society of America*, vol. 34, no. 4, pp. 185-188, 1944.
- [25] J. Alva y M. Escalaya, "Actualización de los parámetros sísmológicos en la evaluación del peligro sísmico en el Perú", Simposio Internacional en Honor al profesor Yuji Ishiyama, Lima, 2005.
- [26] R. Youngs, S. Chiou, W. Silva, and J. Humphrey, "Strong ground motion attenuation relationships for subduction zone earthquakes", *Seismological Research Letters*, vol. 68, no. 1, pp. 58-73, 1997.
- [27] K. Sadigh, C. Chang, J. Egan, F. Makdisi, and R. Youngs, "Attenuation relationships for shallow crustal earthquakes based on California strong motion data", *Seismological research letters*, vol. 68, no. 1, pp. 180-189, 1997.
- [28] Instituto Nacional de Defensa Civil, "Mapa de peligros de la ciudad de Ayacucho", Proyecto INDECI - PNUD PER /02/051 ciudades sostenibles, 2003.
- [29] J. Palomino, "Estudios geofísicos para la zonificación sísmica de la ciudad de Ayacucho", thesis, Univ. Nac. San Cristóbal de Huamanga, Ayacucho, Perú, 2008.
- [30] A. López, "Evaluación geofísica geotécnica del proyecto central hidroeléctrica chilia, región Huanuco-Ancash", thesis, Univ. Nac. San Agustín de Arequipa, Perú, 2016 [Online]. Available: <http://repositorio.unsa.edu.pe/handle/UNSA/2535>
- [31] C. Park, R. Miller, and H. Miura, "Optimum Field Parameters of an MASW Survey", Japanese Society of Exploration Geophysics Extended Abstracts, Japan, 2002.
- [32] C. Cornell, "Engineering seismic risk analysis", *Bulletin of the seismological society of America*, vol. 58, no. 5, pp. 1583-1606, 1968.
- [33] M. Ordaz et al., "Development and validation of software CRISIS to perform probabilistic seismic hazard assessment with emphasis on the recent CRISIS2015", *Computación y Sistemas*, vol. 21, no. 1, pp. 67-90, 2017.
- [34] D. Benedetti and V. Petrini, "Sulla vulnerabilità sismica di edifici in muratura: un método di valutazione. A method for evaluating the seismic vulnerability of masonry buildings", *L'industria delle Costruzioni*, no. 149, pp. 66-74, 1984.
- [35] J. Rodríguez, "Aplicación del método italiano del índice de vulnerabilidad sísmica en viviendas del asentamiento humano El Altillio en el distrito del Rímac", thesis, Univ. Nac. Ingeniería, Lima, Perú, 2005 [Online]. Available: <http://cybertesis.uni.edu.pe/handle/uni/2954>
- [36] F. Yépez, A. Barbat, y J. Canas, *Riesgo, peligrosidad y vulnerabilidad sísmica de edificios de mampostería*. Barcelona, España: Centro Internacional de Métodos Numéricos en Ingeniería (CIMNE), 1995.
- [37] I. Gómez y E. Rodríguez, "Generación de funciones de vulnerabilidad para edificaciones de mampostería no reforzada de baja altura utilizando técnicas de simulación" thesis, Univ. Ind. Santander, Colombia, 2006.
- [38] C. Zavala y R. Proñaño, "Estimación rápida de la Respuesta Sísmica en base a sistemas de un grado de libertad para el cálculo de vulnerabilidad sísmica", in XIV Congreso Nacional de Ingeniería Civil, Iquitos, Perú, 2003.
- [39] H. Scaletti, "Estimación Rápida de Desplazamientos Laterales Producidos por Sismo", in Conf. Internacional de Ingeniería Sísmica, Lima, Perú, 2007.
- [40] E. Miranda, "Approximate seismic lateral deformation demands in multistory buildings", *Journal of Structural Engineering*, vol. 125, no.4, pp. 417-425, 1999.
- [41] S. Grimaz, F. Meroni, V. Petrini, R. Tomasoni, and G. Zonno, "El papel de los datos de daños del terremoto de Friuli en el estudio de modelos de vulnerabilidad sísmica de edificios de mampostería", in Conf. Ciencia y terremotos - Análisis y perspectiva desde la experiencia de Friuli - 1976/1996, Italia, 1996, pp. 89-96.



Los artículos publicados por TECNIA pueden ser compartidos a través de la licencia Creative Commons: CC BY 4.0. Permisos lejos de este alcance pueden ser consultados a través del correo revistas@uni.edu.pe